



PROJECT NO. 33000.21: LAND FORMALIZATION: SPLUMA PROCESS

WATER SERVICES REPORT

PELLA DEVELOPMENT

	Revisions						
No	Date	Compiled	Revised	Description			
0	2019/05/15	Mr. P. Jonker	EM Koegelenberg	WATER SERVICES REPORT			

SUBMITTED BY:

BVi Consulting Engineers April 2019



BVi Regional Office 17A Keerom Street SPRINGBOK 8240

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First Issue Date:	5/15/2019	Compiled by:	P Jonker
Date:	5/15/2019	Revised by:	EM Koegelenberg

REVIEWED BY CLIENT (MARK APPROPRIATE BOX WITH AN "X"):

A	REVIEWED AND ACCEPTED.
В	REVIEWED AND ACCEPTED AS NOTED. WORK MAY PROCEED SUBJECT TO INCORPORATION OF CHANGES AS INDICATED. REVISE & RE-SUBMIT FOR REVIEW.
C	REVISE AND RE-SUBMIT FOR REVIEW. WORK MAY NOT PROCEED.
D	NO FURTHER REVIEW REQUIRED.

CLIENT APPROVAL:

Name:	
Designation:	
Signature:	
Date:	







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1. EXECUTIVE SUMMARY

A report was requested by the Client on the available bulk water capacity for the town Pella. The Client wanted to be informed should limitations exist for the planned developments in order for them to take immediate action to address the limitations.

The existing water infrastructure network requires the following upgrading of works:

Construction of a 360 kl Storage Reservoir

The identified scope of works must be implemented immediately, in order to ensure the new development is possible.

The following upgrade is recommended but not required:

• Installation of an additional pipeline from the reservoir to the area of the new development







2. INTRODUCTION

Vedanta (hereinafter referred to as the "Client") appointed BVi Consulting Engineers (hereinafter referred to as the "Engineer") to assist them with their SPLUMA application processes under their land formalizing processes. A Water Services Report is one of the mandatory requirements under the SPLUMA application. This report will only focus on the Water Service for Pella, Northern Cape.

A report was requested by the Client on the available bulk water capacity. The Client wanted to be informed should limitations exist for the planned developments in order for them to take immediate action to address the limitations. This report will only focus on the available bulk supply capacity limited to concept detail.

4. COMMUNITY PROFILE

4.1 POPULATION DATA

According to information from Khâi-Ma Municipality, the resident figures of Pella are shown in Table 1:

Table 1: Population Data of Pella

Description of Items	Curren	t 2019	Additional Development (118 plots)	
	Plots	Population	Plots	Population
1. Total plots	700	2520	818	2945

There are currently 700 households with a population of 2520 persons in Pella. The planned development will consist of 118 additional plots. With the development taken into account the projected population will be approximately 2945 in the near future.

5. **DESIGN GUIDELINES**

The following design guidelines are adopted to determine the existing and future demands of the town Pella:

Average persons per dwelling unit

3.6

Consumption per person per day (Household)

100 t/c/day







•	Peak factor in summer peak consumption	1.5
•	Water losses in the network	10%
•	Industrial water demand	10%
•	Minimum required pressure during instantaneous peak	24 m

These guidelines and assumptions are based on the following design codes:

- Technical Guidelines for the Development of Water and Sanitation Infrastructure, Second Edition (2004), Department of Water Affairs and Forestry.
- Guidelines for Human Settlement Planning and Design, Building and Construction Technology, Volume 2, CSIR (The Red Book).

6. PROJECTED WATER DEMAND AND PEAK FLOWS

6.1 PROJECTED WATER DEMAND

Table 2 summarises the projection of the current water demand and the water demand after the new development, using the design parameters as discussed earlier. The projected water demand also takes into account the industrial demand of 10%. The water losses in the network are assumed to be approximately 10%.

Table 2: Projected Water Demand

YEAR	POPULATION	OCCUPIED PLOTS	NORMAL WATER DEMAND (KE)	INDUSTRIAL DEMAND AND CONVEYANCE LOSSES: 20 % (KC)	ANNUAL AVERAGE DAILY DEMAND (K&)	SUMMER PEAK PERIOD: WATER DEMAND (KC)
2019	2 520	700	252	50.4	302	454
After development	2 945	818	294	58.9	353	530





6.2 PROJECTED PEAK FLOW

Table 3 summarises the peak flow factor of 1.5 during peak periods in the summer for the current population and the population after the new development.

Table 3: Projected Peak Flow

Year	Projected Peak Flow (€/s)				
i Cai	AADD (ke/day)	Average Flow (e/s)	Peak period flow (e/s)		
2019	302	3.50	5.25		
Future Development	353	4.09	6.14		

7. EXISTING WATER INFRASTRUCTURE

7.1 BULK SUPPLY

The water is abstracted from the Orange River, purified and then pumped by Sedibeng Water Board to Aggeneys. A pipeline is connected to the Sedibeng pipe that is connected to a break-pressure reservoir. The water is distributed from the break-pressure reservoir to the existing municipal reservoir, from where it is distributed to the reticulation network in Pella, by means of gravitational flow.

7.2 RETICULATION NETWORK

The existing reticulation network of Pella consists of a combination of uPVC and HDPE pipelines. All existing households are connected to the existing reticulation network in order to be supplied with potable water.

7.3 STORAGE RESERVOIR

Pella water storage capacity consists of one (1) concrete reservoir with a storage capacity of 350kt. According to the "Guidelines for Human Settlement Planning and Design", 48 hours of water storage is prescribed for bulk storage.







8. EVALUATION OF THE EXISTING WATER SUPPLY SYSTEM

8.1 EXISTING BULK SUPPLY: SEDIBENG WATER BOARD

The existing Water Treatment Plant located at Pelladrift was originally designed for a capacity of 12 MU day. The towns of Pella, Pofadder and Aggeneys are supplied with water from the treatment plant located in Pelladrift. The Client is currently busy expanding the mining activities at Gamsberg Mine which requires additional water and therefore upgraded the Water Treatment Plant located in Pelladrift with an additional capacity of 15MU/day, in order to make provision for the water demand that is caused due to the expansion of the mining activities.

The existing water Treatment Plant located in Pelladrift will be sufficient to provide water for the towns of Pella, Pofadder and Aggeneys.

8.2 EXISTING RETICULATION NETWORK

The existing reticulation network was analysed to determine if any pressure failures are present in the existing water reticulation network of the town Pella. It was determined that certain parts of the high lying areas, especially to the South-West of the town experience low pressures during peak flow periods of less than 20m pressure.

8.3 EXISTING STORAGE CAPACITY

The existing Municipal reservoir located in Pella has a capacity of 350kl. This capacity is insufficient to supply the town of Pella with water for 24 hours during peak demand periods and also to provide the required 48 hours AADD storage capacity.

9. REQUIRED STORAGE CAPACITY

The required storage capacity for the current demand and the demand after the new development is shown in Table 4.

Table 4: Required Storage Capacity

Year	AADD (kℓ/day)	Storage Capacity (ke) (48 Hours Period of AADD)
2019	302	605
Future Development	353	707







As mentioned in section 8.3, the current storage capacity is already insufficient for peak demand periods, e.g. during Summer times. In addition, the current storage capacity is also insufficient to provide storage for 48 hours of AADD. Fortunately, the feed from Sedibeng is relatively reliable and the flow from the Sedibeng pipeline could refill the reservoir during the day in high demand periods.

However, the Guidelines from Human Settlement Planning and design, states that towns must have at least 48 hours AADD storage capacity. Therefore, an additional storage reservoir of at least 360kt will be required to provide sufficient storage for the current demands and accommodate the new development of 118 plots.

10. PLANNED INFRASTRUCTURE

The following scope of works needs to be executed, in order to address the existing shortfalls experienced within the existing bulk supply system of the town Pella and also to make provision for the new 118 plots development.

10.1 BULK STORAGE CAPACITY

For the bulk storage capacity to be able to accommodate the new development, the construction of a 360kt storage reservoir is required.

10.2 INTERNAL RETICULATION NETWORK

As discussed earlier, there are certain areas in the South-West of the town where the pressures drop below 20 m during peak periods. This is also the area where the new development is planned. In an attempt to prevent the pressures dropping below 20 m, it is recommended that a pipeline with a diameter of at least 110mm be provided directly from the reservoir to the new development area. Refer to Figure 1.

11. RECOMMENDATIONS

The findings and conclusion in this report are based on a preliminary desktop study, as well as site visits. The following are required/recommend in order to ensure the new development is possible without providing undue strain on the water network of the town Pella:







Required: Construction of a proposed 360kl storage reservoir.

Recommended: The construction of an additional section of 110mm dia. from the reservoir

directly to the area where the new development will be. Refer to Figure 1.

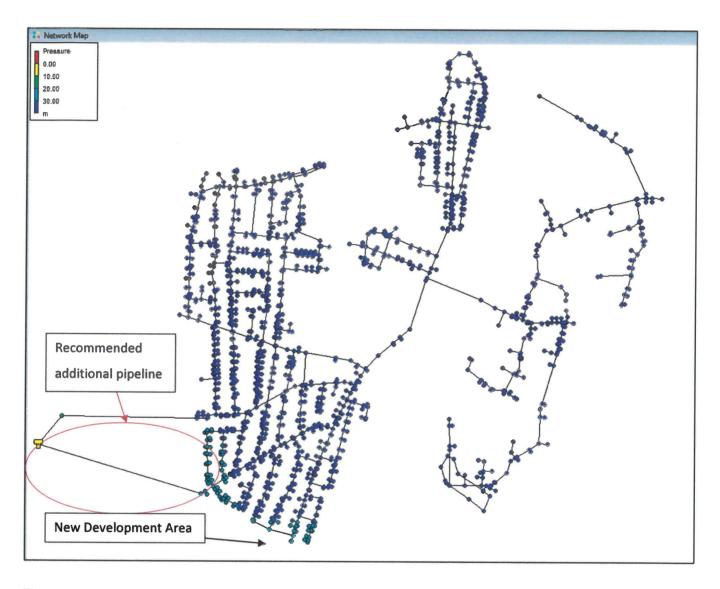


Figure 1: Pella Water Network





12. GENERAL

This report was compiled by BVi Consulting Engineers at the request of Vedanta Resources. Please contact us at this office if any enquiries arise.

Signed by:

EM Koegelenberg Pr. Eng.

.....15/05/2019......

Date





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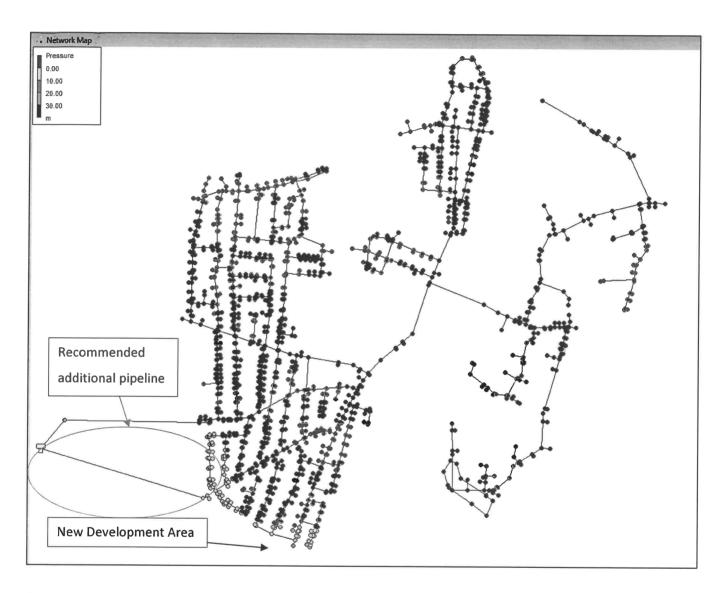


Figure 1: Pella Water Network





12. GENERAL

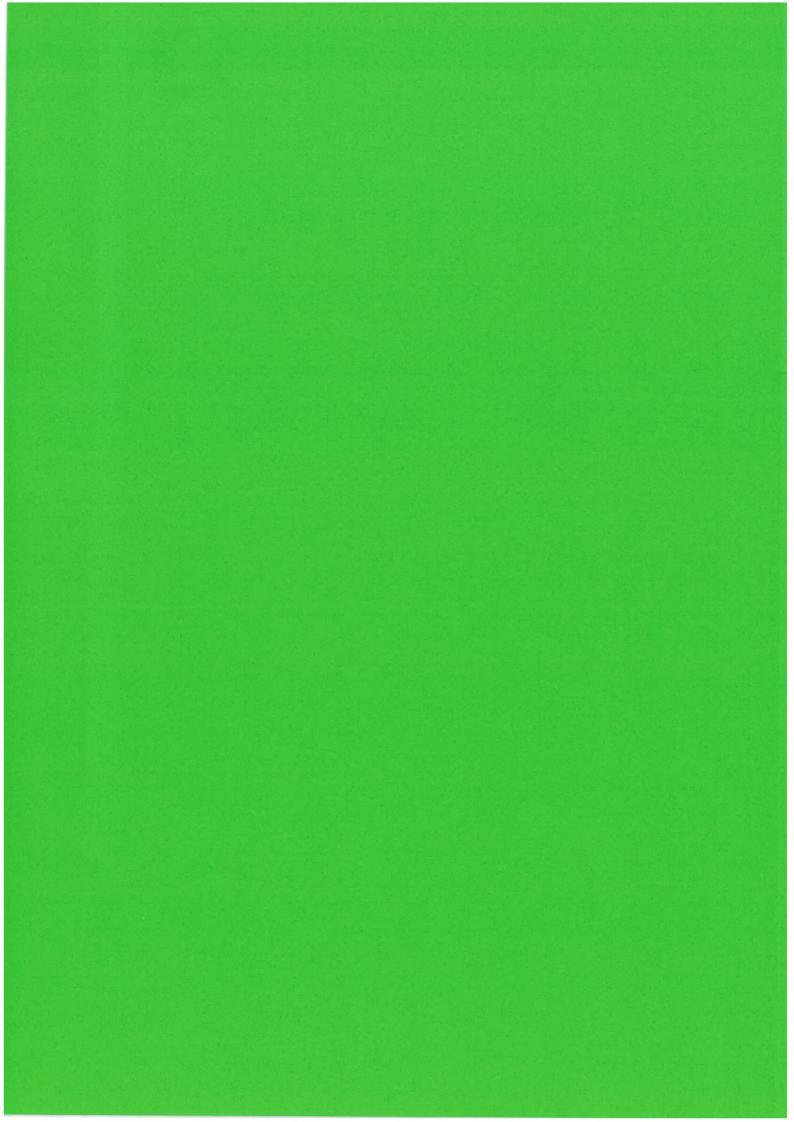
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Signed by:

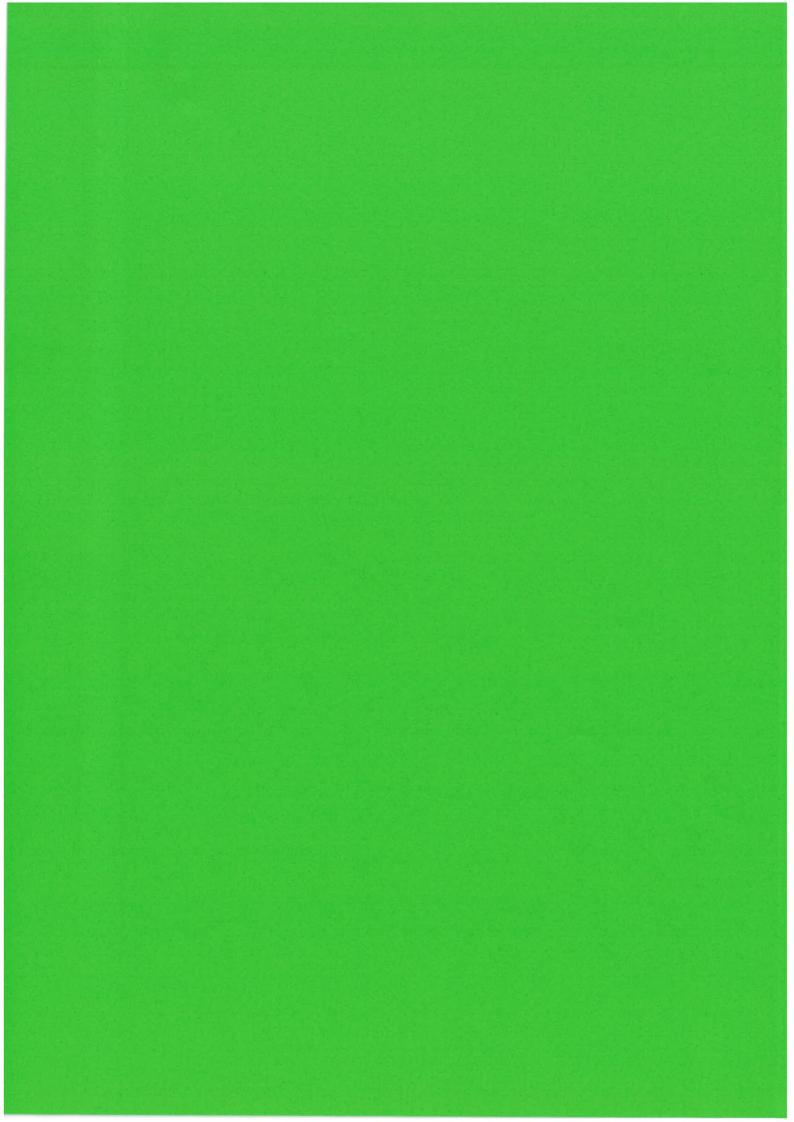
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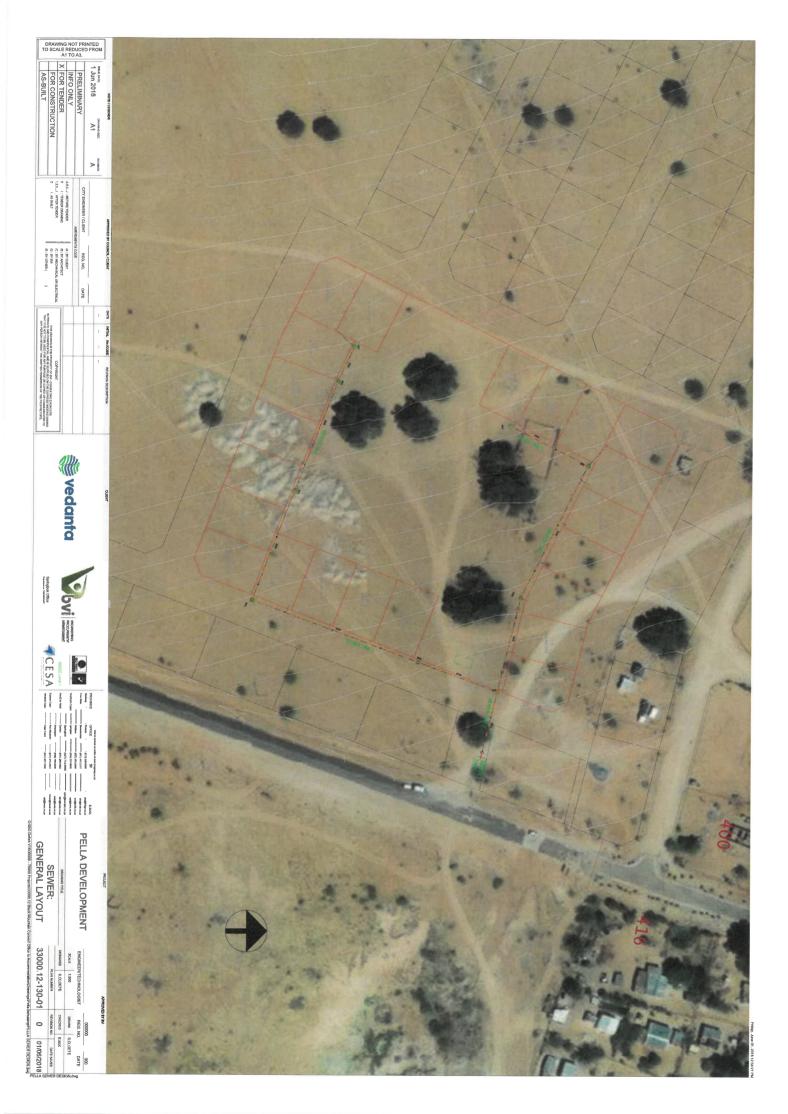
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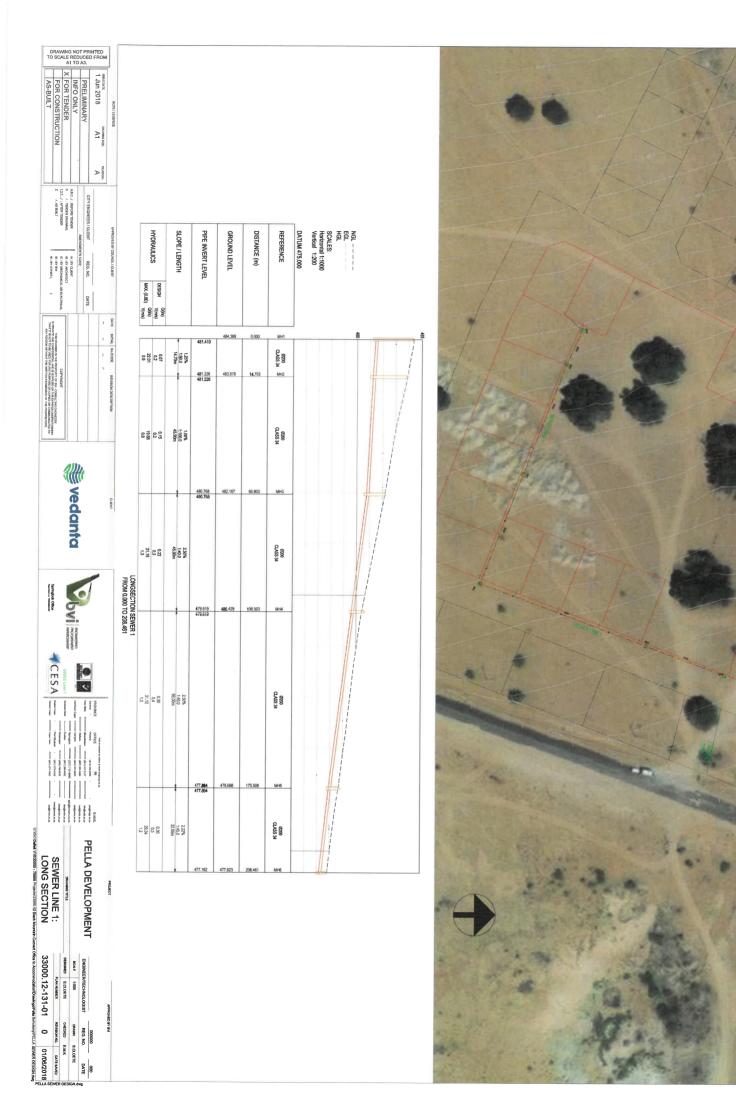
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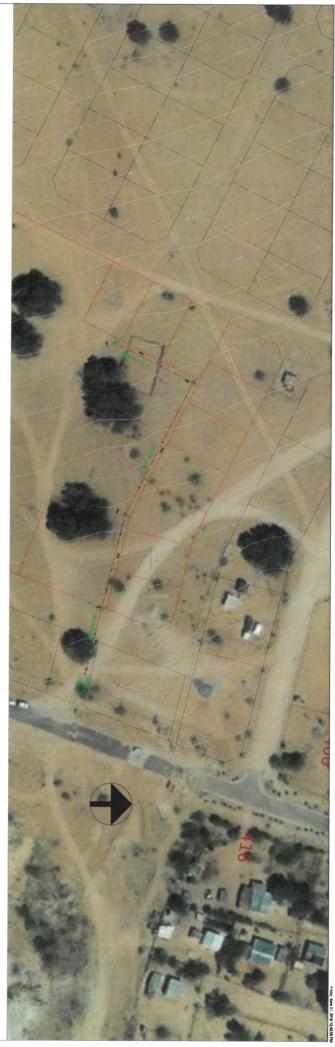


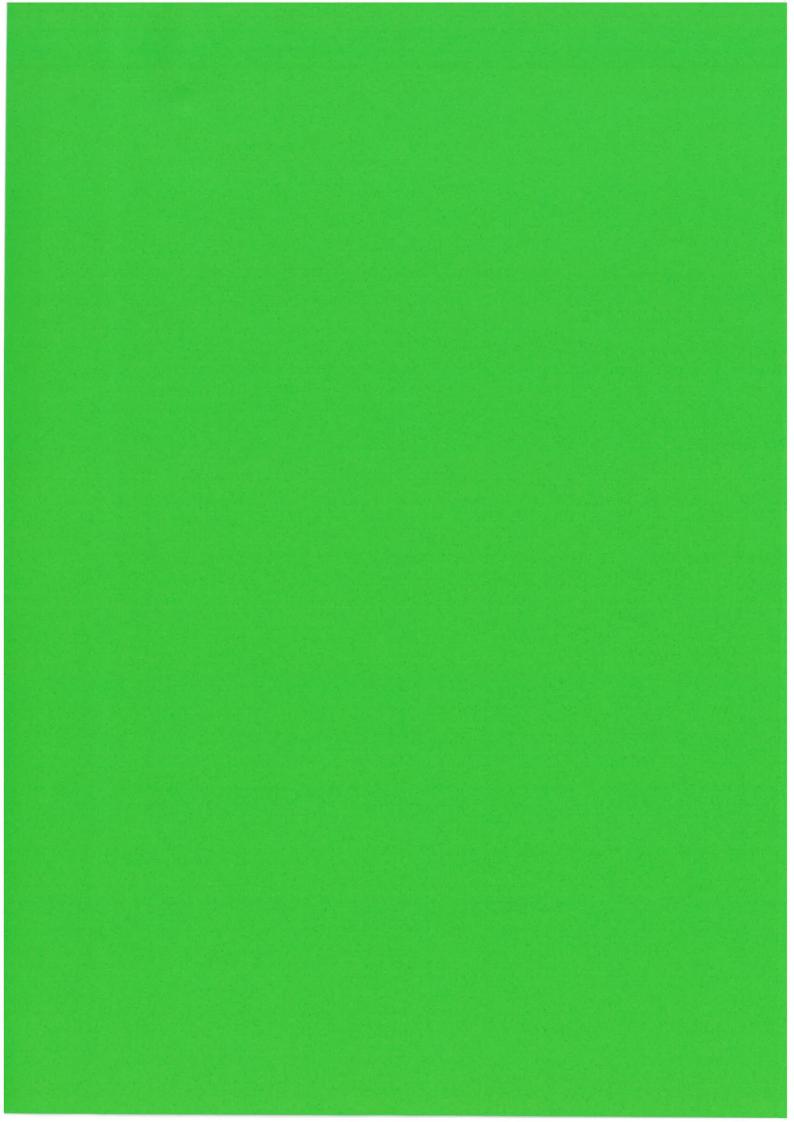




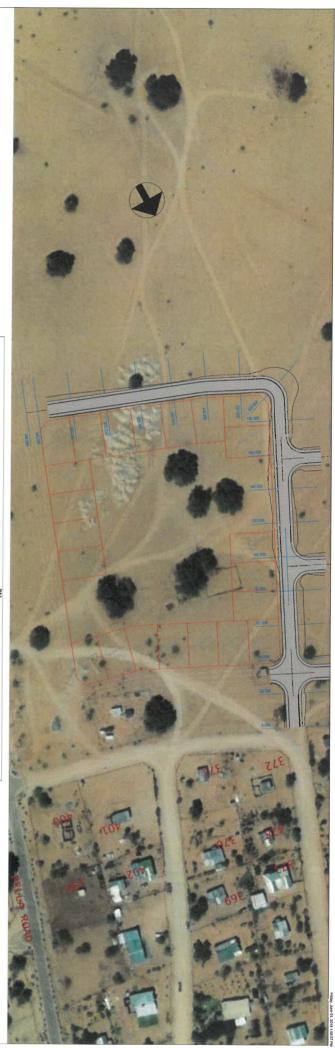


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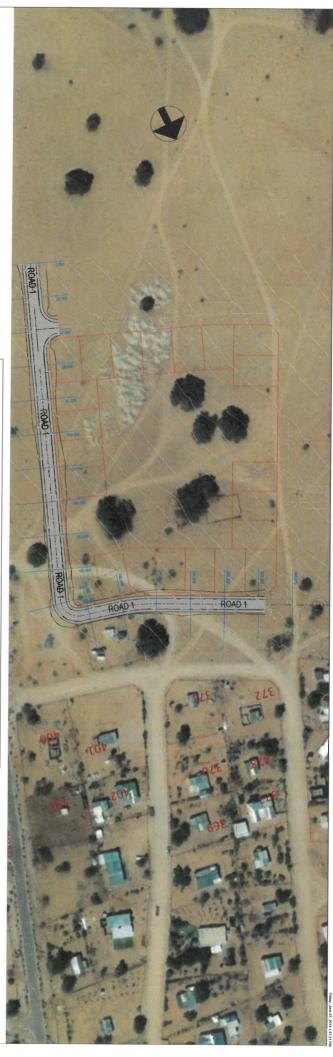




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			R-80			LONGSI FROM	120	DIRECTION 201'06'40"	479.476 479.022 478.519	479.346 478.523 478.324	479,406 478,883 478,384	479.466 478.943 478.444	116.03 46850 PKC 116.03 PKC 116.0		=95.626	
Springboil Office	Parameter M G					FROM 0.000 TO 320.000	180	₩ BCC 	477.960 477.524 477.112	477.838 477.356 477.000	477,898 477,418 477,080	477.476 477.476	MF 1270 1270 1270 1270 1770 1770 1770 1770	EVC2=	=180.499 =185.499 =187.670	
CESA Emmora	Horner Ca	0	PROVINCE	-			220	BCC 280 LRVE 288 CURVE 288 CURVE 288 CURVE 288 CURVE 287	477. 422 478.052	477.234 477.865	477.294 477.925	477.354	222479 67438		-222670	
Duttes (\$10,000-500) 200-500 (\$10,000-500) (\$10,000 (\$10,	Straight		Will or contact us or OFFICE				260	VE 59 C DIRECTION 116'43700' 116'43700'	478.672 479.286	478.501 479.138	478.561 479.198	478.621 479.258	7,075	1000		
(81) 20-632 — Bully-for-on at (90) 78-620 — Bully-for-on at (91) 573-542 — Bully-for-on at (91) 573-542 — Spilly-for-on at (91) 573-642 — Spilly-for-on at (92) 573-642 — Spilly-for-on at (93) 573-642 — Spilly-for-on at (94) 573-642 — Spilly-for-on at (94	(227) 712-0000 pp@sveu.n.m		WW-3 E-WWT				300	THON STOP	479.923 480.569	479.773	479.833 480.433	479,893	200 200 200 200 200 200 200 200 200 200	BVC4=	294.887	P4).
STREET 1:		PELLA DEVELOPMENT		PROJECT												0
33000	SCALE	ENGINEER/T														



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