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## Attention: Mr Pieterse

## **Report on Borehole Drilling and Aquifer Test: Klaarstroom**

#### 1. Introduction

SRK Consulting (South Africa) (Pty) Ltd (SRK) was appointed by the Prince Albert Municipality conduct a geohydrological investigation at Klaarstroom. The exploration Phase consisted of the following:

- Borehole Siting;
- Borehole Drilling; •
- Aquifer Test; and •
- Water Quality Test. •

#### 2. Drilling

Gobora Drilling was appointed to drill the borehole at Klaarstroom. Drilling started on 26 May 2017 and was completed on 19 June 2017.

The details of the borehole construction are presented in Table 1 and position of the borehole in Figure 1.

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#### Table 1: Borehole Construction Details

BH 1						
BH Depth (m)	BH Diameter (mm)	Casing Type and Diameter	Blow Yield (L/s)	Rest Water Level (m bgl)		
0-6	254	219 mm steel casing				
6-85	208	Open hole	0.5	7.64		
85-250	171	Open hole				
		BH 2				
BH Depth (m)	BH Diameter (mm)	Casing Type and Diameter	Blow Yield (L/s)	Rest Water Level (m bgl)		
0-6	254	219 mm steel casing				
6-70	208	Open hole	1.5	6.86		
70-250	171	Open hole				
		BH 3				
BH Depth (m)	BH Diameter (mm)	Casing Type and Diameter	Blow Yield (L/s)	Rest Water Level (m bgl)		
0-6	254	219 mm steel casing		8.31		
6-85	208	Open hole	0.2			
85-250	171	Open hole				
		BH 4				
BH Depth (m)	BH Diameter (mm)	Casing Type and Diameter	Blow Yield (L/s)	Rest Water Level (m bgl)		
0-6	254	219 mm steel casing				
6-80	208	177 mm steel casing (30m perforated)	0.2	22.21		
80-250	171	Open hole				
		BH 5				
BH Depth (m)	BH Diameter (mm)	Casing Type and Diameter	Blow Yield (L/s)	Rest Water Level (m bgl)		
0-6	254	219 mm steel casing				
6-80	208	177 mm steel casing (30m perforated)	0.2	10.18		
80-250	171	Open hole				
BH 6						
BH Depth (m)	BH Diameter (mm)	Casing Type and Diameter	Blow Yield (L/s)	Rest Water Level (m bgl)		
0-6	254	219 mm steel casing				
6-85	208	Open hole	3	8.65		
85-250	171	Open hole				



#### Figure 1: Klaarstroom drill position

## 3. Borehole Testing Results

The pumping tests included step drawdown and constant discharge tests with subsequent recovery monitoring after each test. The purpose of the step-test is to establish the efficiency of a single borehole and to provide preliminary information on the yield of the borehole, both from a quantitative and qualitative perspective. The purpose of the constant discharge test is to determine the hydraulic properties of the aquifer adjacent to the tested borehole and to investigate, identify and characterise nearby hydraulic boundaries. This data, together with the recovery test results, is used to determine the optimal and safe yield and pumping schedule of the borehole.

## KS BH 1

## **Step Discharge Test**

A total of three steps of 60 min duration each were conducted on borehole KS BH1 (**Table 2 and Figure** 2). The associated pumping rates varied between 0.57 and 1.07 L/s. **Figure 2** presents the water level in the borehole when pumped at these different rates, and the recovery response when the pump was switched off. The effective drawdown in the borehole was 92.74 m after 7 min into final step. The water level in the borehole recovered to 0.91 m after 180 min of pump shut-down, i.e. a 99 per cent recovery.

	Step Drawdown Tes	st	Constant Discharge Test			
Step	Pumping Rate (L/s)	Drawdown (m)	Pumping Rate Time (L/s) (mins)		Drawdown (m)	
Step 1	0.57	25.66	0.25	2 520	28.19	
Step 2	0.76	82.74	Recovery	210	0.59	
Step 3	1.07	92.74		·		
			Notes:			
			Receiver to 07% of drawdown			
Recovery	180 mins	0.91 (99%)	Recovery to 97% of drawdown.			

Table 2: Details of Pumping Tests Conducted on BH 1

## **Constant Discharge Test**

Based on the results of the step drawdown test, it was decided to undertake a constant discharge test at c.0.25 L/s. The borehole was pumped for 42 hours with a resultant drawdown of 28.19 m. The results are presented as a log-normal graph in **Figure 3**. The borehole recovered to 0.59 m 210 min after pump shutdown before recovery was stopped.



Figure 2: Results of Step Drawdown Test



Figure 3: Results of Constant Discharge Test

## KS BH 6

## **Step Discharge Test**

A total of seven steps of 60 min duration each were conducted on borehole **KS BH6** (**Table 3 and Figure 4**). The associated pumping rates varied between 0.52 and 5.02 L/s. **Figure 4** presents the water level in the borehole when pumped at these different rates, and the recovery response when the pump was switched off. The effective drawdown in the borehole was 65.5 m after 50 min into final step. The water level in the borehole recovered to 2.11 m after 600 min of pump shut-down, i.e. a 96 per cent recovery.

Table 3: Details of Pumping Tests Conducted on KS BH 6	

	Step Drawdown Tes	st	Constant Discharge Test			
Step Pumping Rate (L/s)		Drawdown (m)	Pumping Rate (L/s)	Time (mins)	Drawdown (m)	
Step 1	0.52	1.78	2.02	1 800	43.92	
Step 2	0.75	2.89	Recovery	1 560	2.05	
Step 3	1.01	4.49				
Step 4	1.51	6.84				
Step 5	2.01	9.34	Notes:			
Step 6	3.01	17.64				
Step 7 5.02		65.5	Recovery to 95% of drawdown.			
Recovery	600 mins	2.11 (96%)				

# **Constant Discharge Test**

Based on the results of the step drawdown test, it was decided to undertake a constant discharge test at c.2 L/s. The borehole was pumped for 30 hours with a resultant drawdown of 43.92 m. The results

are presented as a log-normal graph in **Figure 3**. The borehole recovered to 2.05 m 1 560 min after pump shutdown before recovery was stopped.



Figure 4: Results of Step Drawdown Test



Figure 5: Results of Constant Discharge Test

## 4. Sustainable Yield

To estimate optimum pumping rates, pumping schedules and aquifer parameters, the test pumping data were analysed by means of an Excel based software package developed by Van Tonder et al

(2002). In the software package, various methods such as the Flow Characteristic method (FCmethod), porous aquifer solutions (Theis, Cooper-Jacob and Hantush methods), fractional pumping test analysis (Barkers Generalised Radial Flow Model) and step drawdown analysis were used to estimate risk-based sustainable yields for the boreholes as well as aquifer parameters such as transmissivity (T) and the storage coefficient (S). In the FC-Analysis the following aquifer input parameters were used:

- Effective recharge of 5 mm per annum.
- Data were extrapolated for 2 year.

The summary of the results are presented in the Tables below.

Method	Sustainable Yield (L/s)	Std. Dev	Early T (m²/d)	Late T (m²/d)	Available Drawdown Used
Basic FC	0.13	0.08	0	0.2	90
FC Inflection point	0.22	0.01			
Cooper Jacob	0.3	0.19			35
Average Yield (L/s)	0.22	0.09			
Recomme					
Hours per day of	24	0.22 L/s	NOTES: Pump to be installed at 100 m bgl.		
pumping at rate of.	16	0.25 L/s			

Based on the single well test results, it is recommended that borehole BH1A may be pumped as follows:

- 0.22 L/s for 24 h/day (18 m<sup>3</sup>/day); or
- 0.25 L/s for 16h/day (14m<sup>3</sup>/day).

Borehole KS BH1 can be used to irrigate the sportsfield and should be pumped into tanks.

## Table 5: Recommended Yield for Borehole KS BH 6

Method	Sustainable Yield (L/s)	Std. Dev	Early T Late T A (m²/d) (m²/d)		Available Drawdown Used	
Basic FC	0.5	0.36	3	0.7	90	
FC Inflection point	1.84	0.09			35	
Cooper Jacob	0.60	0.39		0.8	90	
Average Yield (L/s)         0.94         0.68		0.68				
Recomme						
Hours per day of	24	1 L/s	NOTES: Pump to be installed at 100 m bgl.			
pumping at rate of.	12	1.5 L/s				

Based on the single well test results, it is recommended that borehole KS BH 6 may be pumped as follows:

- 1 L/s for 24 h/day (86 m<sup>3</sup>/day); or
- 1.50 L/s for 12h/day (65 m<sup>3</sup>/day).

The daily limit for registering the borehole is 20m<sup>3</sup> and if more water is abstracted, the water use must be licensed.

## 5. Water Quality

Presented in **Table 6** are a summary of the water quality results. The laboratory certificates are listed in Appendix A. The results show that the EC of groundwater is high and mainly due to the elevated Chloride concentrations. It is also important that the water quality be compared to the existing borehole water quality.

The analyses indicated *E.coli* of 19 count/100ml at BH2 and 1 count/100ml at BH 3. The groundwater should be regularly monitored to detect if microbiological counts does not increase. The water should also be treated before use.

Constituent	BH 1	BH 2	BH 6
pH (at 25°C)	7.0	7.2	7.2
Conductivity (at 25°C) (mS/m)	<mark>351.4</mark>	<mark>193.2</mark>	<mark>274.3</mark>
Turbidity	0.62	<mark>12.33</mark>	7.38
	mg/l	mg/l	mg/l
Fluoride (mg/l)	0.0	0.1	0.0
Calcium (as Ca) (mg/l)	131.4	94.3	135
Magnesium (as Mg) (mg/l)	81.8	56.2	135
Sodium (as Na) (mg/l)	347.9	193.2	274.3
Potassium (as K) (mg/l)	3.4	3.6	3.7
Chloride (as Cl) (mg/l)	<mark>790</mark>	<mark>337</mark>	<mark>515.8</mark>
Sulphate (as SO <sub>4</sub> ) (mg/l)	234	117	173
Ammonia Nitrogen (as N) (mg/l)	<0.28	0.4	0.38
Nitrate & Nitrite Nitrogen (as N) (mg/l)	0.02	0.05	0.03
Iron (as Fe) (mg/l)	0.1	0.1	0.1
Manganese (as Mn) (mg/l)	0.16	0.54	0.56

#### Table 6: Water quality results

# 6. Recommendations

The recommendations arising from the exploration Phase of the project are:

#### BH 1

- The borehole can be pumped at 0.22 L/s for 24 h/day (18 m<sup>3</sup>/day) or 0.25 L/s for 16h/day (14 m<sup>3</sup>/day).
- Pump to be installed at 100 mbgl;
- Groundwater to be used for irrigating the sports field.

## BH 6

- The borehole can be pumped at **1** *L*/s for **24** *h*/day (86 m<sup>3</sup>/day) or **1.50** *L*/s for **12***h*/day (65 m<sup>3</sup>/day).
- Pump to be installed at 100 mbgl;
- Groundwater to be treated before use.

## Additional Recommendations

- A flow meter to measure total water use should be installed;
- An automatic datalogger should be installed to record the water level;
- A low-level cut-off switch should be installed 5 m above the pump intake; and
- The water use needs (not exceeding 20 m<sup>3</sup>/d) to be registered with the DWS (*For assistance with the water use registration you may contact SRK for a quotation*);

# **Prepared by**

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