

TOPLINE 248 HOUSING DEVELOPMENT

Engineering Services Investigation Report

Investigation of the available and required bulk civil and electrical services for the Topline village development in the !Kheis municipal area

AUGUST 2020

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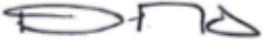
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03	2020/08/25	Draft report to be circulated to relevant parties.	F.D. MARITZ	F.D. Maritz (Pr.Eng)

APPROVAL:

Author signature		Approver signature	
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EXECUTIVE SUMMARY

This report was compiled to investigate the bulk infrastructure serving the Topline village and to determine whether the bulk infrastructure is adequate for the development of an additional 248 stands, through a low-cost housing development.

The bulk engineering services report includes the following categories:

- Bulk Water Infrastructure
- Bulk Sewer Infrastructure
- Bulk Road and Storm Water Infrastructure
- Bulk Electrical Infrastructure

After investigating the infrastructure, it was found that the existing bulk infrastructure is not sufficient to accommodate the Topline 248 Houses project. The bulk services for each category that require attention before the project can commence is summarised below:

- **Bulk Water Infrastructure**

Upgrading of the entire bulk water supply system is required as these 248 houses will almost double the demand related to the existing 288 houses.

- **Bulk Sewer Infrastructure**

Construction of two new pump stations (6.6 l/s x 2).
Construction of two new 110mm rising mains (1.3km and 2.1km).
Construction of a new 0.5ML waste water treatment works;

- **Bulk Electrical Infrastructure**

Upgrading and extension of the existing bulk electrical supply system is required by Eskom, the extension of the electrical system will not be a problem as the main sub-station in Groblershoop is currently being upgraded and will be commissioned in December 2020

This report can be used both for business plans and funding applications from the various funding schemes available.



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1. INTRODUCTION

1.1 Disclaimer

This is a draft report and only outlines some of the findings of the investigation to date and should not be used as the final or complete report. No recommendations or conclusions have been made and some portions of the report may be incomplete as the investigation is still in process.

1.2 Terms of Reference

- I. BVI Consulting Engineers was appointed by Macroplan to undertake this Bulk Engineering Services Study (Water, Sewer, Electricity and Roads & Storm Water) for the proposed Topline 248 housing project. Topline is one of six villages located close to the Orange river within the jurisdiction of !Kheis Local Municipality.

1.3 Site Location

- I. The site is situated approximately 17.7 km to the north-west of Groblershoop in the Northern Cape (Figure 1 – Locality Plan).
- II. The development is located at the following coordinates: 28°45'13.83" S; 21°50'30.12" E

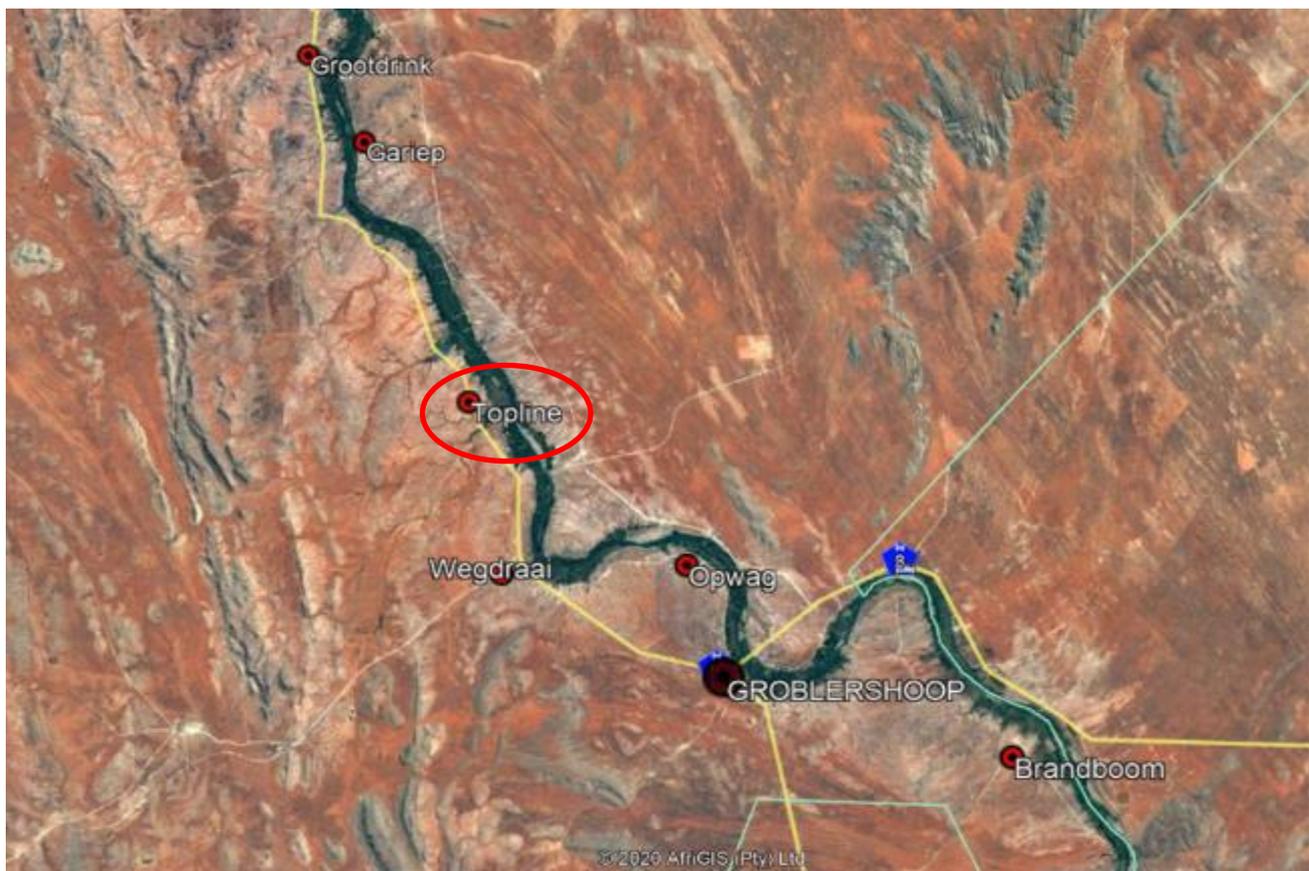


Figure 1: Topline 248 Housing Development Locality Plan

III. The planned development consists of 248 low-cost houses next to the existing village (Figure 2: 248 Stands Development Area)

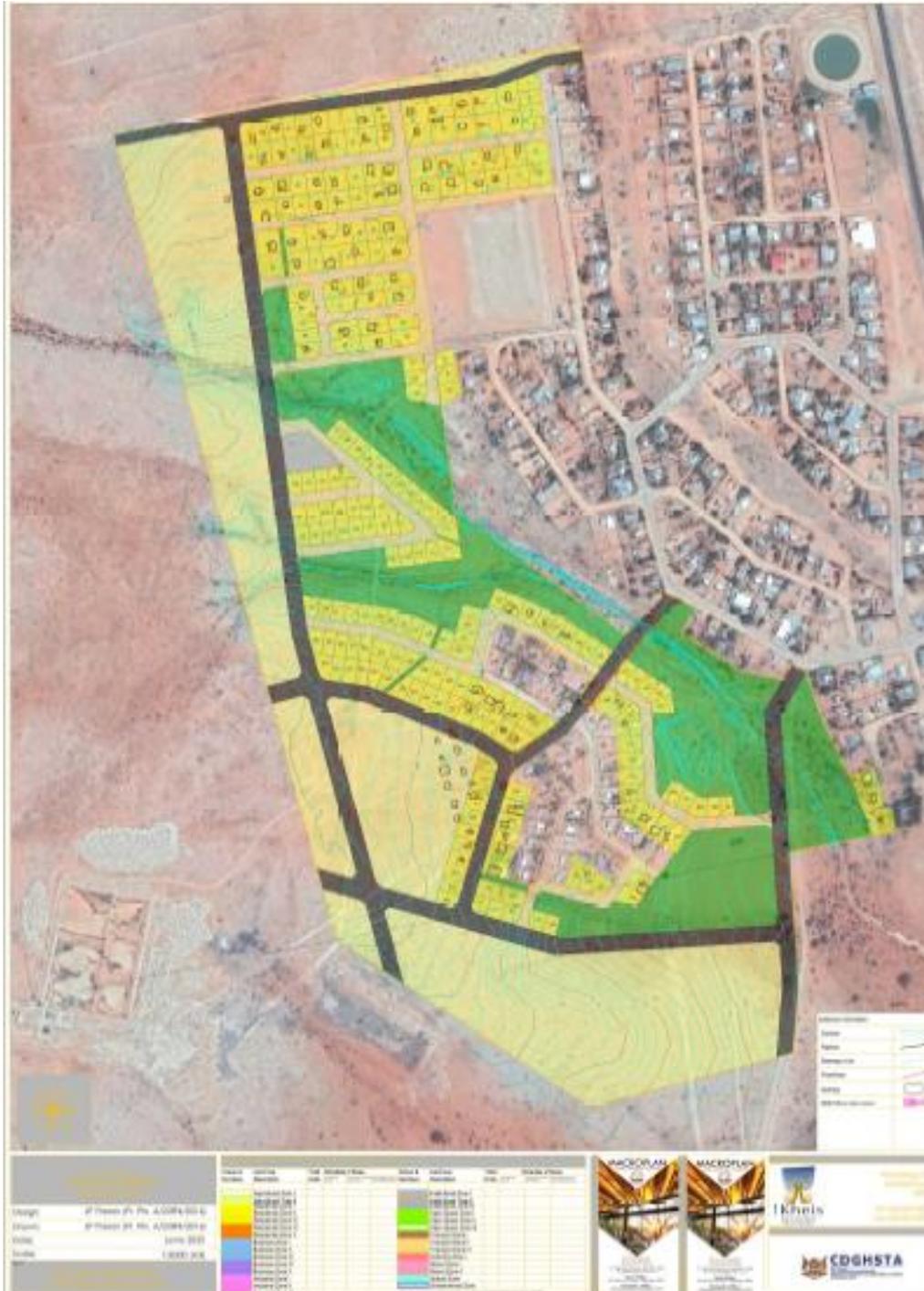


Figure 2: Topline 248 Housing Development Locality Plan

IV. The purpose of the Bulk Engineering Services Assessment is to determine the availability and capacity of existing bulk services to service the proposed development. This report presents the



findings of a preliminary visual inspection and desktop investigation relating to bulk services and further sets out the criteria and standards for the internal services for the new development.

V. The Bulk Engineering Services addressed in this report are the following:

- Water Supply
- Sewerage
- Roads and Access
- Storm Water Management
- Electricity Supply



2. TOPOGRAPHY

The physical characteristics of the site can be summarized as follows:

- Ground cover comprises mostly of natural veld with short grass;
- Topographically, the site has a relatively gentle sloping terrain from the middle of the village
- Calcrete is close to the surface of the natural ground level, which makes excavations very hard.

3. WATER SUPPLY

3.1 Existing Water Infrastructure

Overview

The bulk water infrastructure supplying Topline village with water can be summarised as follows:

- A raw water canal pump station delivering 6l/s;
- A 1500m long, 75mm diameter PVC Class 6 raw water supply line between the canal and the water purification works on the side of the village
- The water treatment works consisting of:
 - A 2500m³ open raw water storage dam
 - A 6.6l/s package type water treatment plant,
 - A 850m³ Sectional steel storage tank
 - A 10l/s high lift pump station
 - A 150m³ Sectional steel elevated reservoir
- Distribution into the village

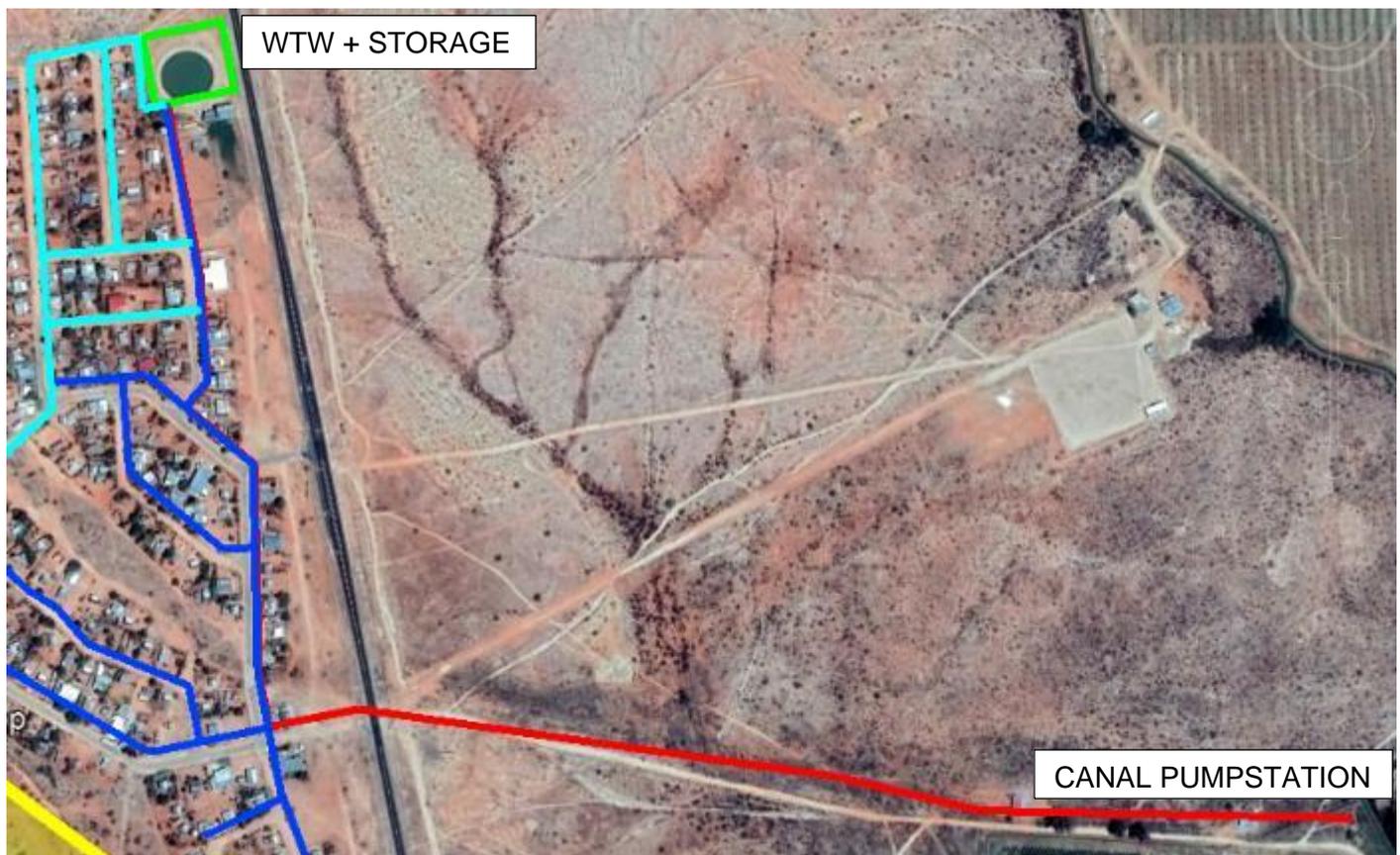


Figure 3: Existing Bulk Water Infrastructure

3.2 Raw Water Supply

Water supplied to Topline is extracted from the canal by means of a secured pump station with a fitted switchgear. The pump station consists of one pump that delivers 6l/s.

Raw water is pumped from the rivier pump station to the purification plant, delivering a maximum flow rate of 6l/s through a 1500m long, 75mm diameter Class 6 PVC pipeline to a 2500 m³ raw water storage dam next to the Package Plant Water Treatment Works in the village



3.3 Water treatment and storage site

The drawing below shows the site layout where the treatment works, raw and potable water storage reservoirs, as well as the pressure tower, is located.

The photo's below shows the reservoirs and treatment plant.



Figure 4: The reservoirs and treatment plant

Water is pumped from the 2500m³ raw water storage dam through the Water Treatment Plant to a 850 m³ sectional steel potable water storage reservoir. From there, it is pumped into the 150m³ elevated storage tank before is gravitates into the village network.

The diagram below shows a schematic layout of the treatment works, raw and potable water storage reservoirs, as well as elevated tower.

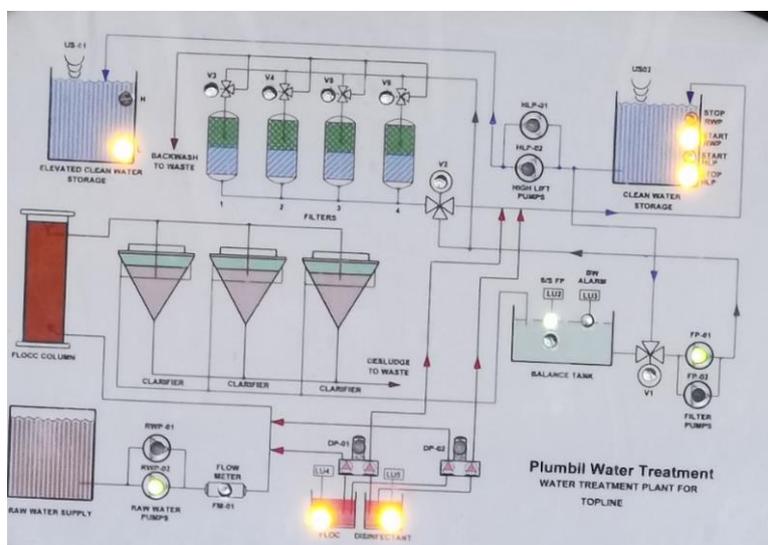


Figure 5: Schematic layout

3.4 Water Treatment Plant

The Package Plant Water Treatment Works (WTW) was constructed in 2008 to supply water at a rate of **2 l/s.**



Photo's below shows the settlement tank, as well as the filters inside the container:





3.5 Reticulation System

The potable water is delivered from the elevated storage tank into the reticulation network via a 75mm diameter PVC Class 6 pipeline. The reticulation network is shown in the drawing below.



Figure 5 The reticulation network

3.6 Condition of the water supply system

Most of the elements of the water supply system are currently manually operated. These include the canal pump, the water treatment works, and the reservoir levels. The elevated tank is functional but leaking heavily. Most of the water meters and pressure gauges are out of service.



3.7 Current water demands and capacity of the existing bulk water supply system

The Red Book was used as basis for calculations of the theoretical capacity for the current bulk water supply system as well as required infrastructure.

The table below shows factors capacities and operating hours used in the calculations:

FACTORS	1	Design Loss Factor Water treatment works (LFw)	10.0%
	2	Design Loss Factor Total conveyance losses (LFr)	15.0%
	3	Summer peak factor (SPF)	1.5
	4	Peak factor reticulation (PFR) From Red Book (Instantaneous Peak)	7
OPERATING HOURS	1	Source Pump Station (SPSH) (Maximum operating hours per day that required volume of water need)	16 hours
	2	Water purification plant (WTPH) (Maximum operating hours per day that required volume of water need)	16 Hours
	3	Lifting Pump Station (LPS%) (% of Instantaneous peak flow)	150%
STORAGE	1	Storage in elevated tanks (Hours of Instantaneous Peak Demand)	4 hours
	2	Potable Water Storage Reservoirs (Hours of Annual Average Daily Demand*SPF)	48 hours
	3	Raw Water Storage Reservoirs (Hours of Summer Average Daily Demand)	7 days

The table on the next page shows the current theoretical demands and capacity of the existing bulk water infrastructure:



BULK AND CONNECTOR SERVICES CAPACITY CALCULATION : CURRENT										
GENERAL	NO.	DESCRIPTION	UNITS		DEMAND PER UNIT		Criteria			
	1	Sub-Economical Houses (Existing)	288	Houses x	600	l household per day	172.8	m ³ /d		
	2	Sub-Economical Houses (135 houses development)		Houses x	600	l household per day	0	m ³ /d		
	4	Economical Houses (Existing)	0	Houses x	1200	l household per day	0	m ³ /d		
	5	Economical Houses (135 houses development)	0	Houses x	1200	l household per day	0	m ³ /d		
	7	Primary School Hostel	0	Learners	150	l Learner per day	0	m ³ /d		
	8	Schools (1)	200	Learners	25	l Learner per day	5	m ³ /d		
	9	High School Hostel	0	Learners	150	l Learner per day	0	m ³ /d		
	10	High School	0	Learners	25	l Learner per day	0	m ³ /d		
	11	Clinics (1)	250	m ² x	500	l/100m ² per day	1.25	m ³ /d		
	12	Businesses, Government and Municipal (1)	6300	m ² x	400	l/100m ² per day	25.2	m ³ /d		
	13	Developed Parks, Sportsgrounds and Day Cares(1)	1.50	ha	5	mm water per day	75	m ³ /d		
	ANNUAL AVERAGE DAILY DEMAND (AADD)							279.25 m³/d		
	THEORETICAL DEMANDS	1	Annual Average Daily Demand (AADD)	AADD		279.3 m ³ /day	11.6 m ³ /hour	3.2 l/s	CURRENT CAPACITY	
2		Gross Annual Average Daily demand (GAADD)	(1+Lfr)*AADD		321.1 m ³ /day	13.4 m ³ /hour	3.7 l/s			
3		Summer Gross Daily Demand (SGDD)	SPF*GAADD		481.7 m ³ /day	20.1 m ³ /hour	5.6 l/s			
4		Instantaneous Peak Demand (IPD) (Main supply pipeline to reticulation)	AADD*PFR			81.4 m ³ /hour	22.6 l/s			
5		Storage Capacity Elevated Storage	hours*IPD				325.8 m ³	150.0 m ³	46%	
6		Lifting Pump Station Capacity and Pipeline Flow between Main Storage and Elevated tank	IPD*LPS%		208 mm dia	122.2 m ³ /hour	33.9 l/s	10.0 l/s	29%	
7		Potable Water Storage Capacity (Main Storage)	hours*AADD				558.5 m ³	850.0 m ³	152%	
8		Water Treatment Plant Capacity (WTPC)	SGDD*24/WTPH		722.6 m ³ /day	30.1 m ³ /hour	8.4 l/s	6.6 l/s	79%	
9		Source Pump Station Capacity and Pipeline Flow	WTPC*(1+LFW)*24/SF		136 mm dia	51.9 m ³ /hour	14.4 l/s	6.0 l/s	42%	
10		Raw Water Storage Capacity	Days*SGDD				3372.0 m ³	2528.0 m ³	75%	

It is clear from the table that the existing infrastructure is already under pressure to handle the demand. Water from the raw water storage dam is also used to irrigate the sportsfield. The biggest problems are with bulk and elevated storages.



3.8 Bulk Water Infrastructure Requirements

The table below shows factors capacities and operating hours used in the calculations for future demand:

FACTORS	1	Design Loss Factor	Water treatment works (LFw)	10.0%
	2	Design Loss Factor	Total conveyance losses (LFr)	15.0%
	3	Summer peak factor (SPF)		1.5
	4	Peak factor reticulation (PFR) From Red Book (Instantaneous Peak)		6
OPERATING HOURS	1	Source Pump Station (SPSH)	(Maximum operating hours per day that required volume of water need	16 hours
	2	Water purification plant (WTPH)	(Maximum operating hours per day that required volume of water need	16 Hours
	3	Lifting Pump Station (LPS%)	(% of Instantaneous peak flow)	150%
STORAGE	1	Storage in elevated tanks	(Hours of Instantaneous Peak Demand)	4 hours
	2	Potable Water Storage Reservoirs	(Hours of Annual Average Daily Demand*SPF)	48 hours
	3	Raw Water Storage Reservoirs	(Hours of Summer Average Daily Demand)	7 days

The table below compares the current infrastructure capacities with the capacity that is required for the 248 stands development. Cells highlighted in red would require upgrading in order to accommodate the expected demands.



BULK AND CONNECTOR SERVICES CAPACITY CALCULATION : FUTURE													
GENERAL	NO.	DESCRIPTION	UNITS		DEMAND PER UNIT			Criteria					
	1	Sub-Economical Houses (Existing)	288	Houses x	600	l	household per day	172.8	m ³ /d				
	2	Sub-Economical Houses (135 houses development)	248	Houses x	600	l	household per day	148.8	m ³ /d				
	4	Economical Houses (Existing)	0	Houses x	1200	l	household per day	0	m ³ /d				
	5	Economical Houses (135 houses development)	0	Houses x	1200	l	household per day	0	m ³ /d				
	7	Primary School Hostel	0	Learners	150	l	Learner per day	0	m ³ /d				
	8	Schools (1)	200	Learners	25	l	Learner per day	5	m ³ /d				
	9	High School Hostel	0	Learners	150	l	Learner per day	0	m ³ /d				
	10	High School	0	Learners	25	l	Learner per day	0	m ³ /d				
	11	Clinics (1)	250	m ² x	500	l	/100m ² per day	1.25	m ³ /d				
	12	Businesses, Government and Municipal (1)	6300	m ² x	400	l	/100m ² per day	25.2	m ³ /d				
	13	Developed Parks, Sportsgrounds and Day Cares(1)	1.50	ha	5	mm	water per day	75	m ³ /d				
	ANNUAL AVERAGE DAILY DEMAND (AADD)								428.05	m³/d			
	THEORETICAL DEMANDS	1	Annual Average Daily Demand (AADD)	AADD	428.1	m ³ /day	17.8	m ³ /hour	5.0	l/s	CURRENT CAPACITY		
2		Gross Annual Average Daily demand (GAADD)	(1+Lfr)*AADD	492.3	m ³ /day	20.5	m ³ /hour	5.7	l/s				
3		Summer Gross Daily Demand (SGDD)	SPF*GAADD	738.4	m ³ /day	30.8	m ³ /hour	8.5	l/s				
4		Instantaneous Peak Demand (IPD) (Main supply pipeline to reticulation)	AADD*PFR			107.0	m ³ /hour	29.7	l/s				
5		Storage Capacity Elevated Storage	hours*IPD					428.1	m ³	150.0	m ³	35%	
6		Lifting Pump Station Capacity and Pipeline Flow between Main Storage and Elevated tank	IPD*LPS%		238	mm dia	160.5	m ³ /hour	44.6	l/s	10.0	l/s	22%
7		Potable Water Storage Capacity (Main Storage)	hours*AADD					856.1	m ³	850.0	m ³	99%	
8		Water Treatment Plant Capacity (WTPC)	SGDD*24/WTPH		1107.6	m ³ /day	46.1	m ³ /hour	12.8	l/s	6.6	l/s	51%
9		Source Pump Station Capacity and Pipeline Flow	WTPC*(1+LFW)*24/SP		168	mm dia	79.6	m ³ /hour	22.1	l/s	6.0	l/s	27%
10		Raw Water Storage Capacity	Days*SGDD					5169.0	m ³	2528.0	m ³	49%	

Recommended upgrades to the Topline bulk water infrastructure are as follows (shown on the drawing below):

- Upgrading of the canal pump station with a duty and standby pump to supply 22l/s.
- New 160 mm diameter Class 6 PVC pipeline between the canal pump station and the existing raw water storage reservoir.
- Upgrading of the raw water storage dam to 5000 m³
- Upgraded Water Treatment Works capable of delivering 46m³/h on the existing treatment works site
- A new 430m³ sectional steel pressure tower on the highest point to the north.
- A new 45l/s uplifting pump station at the treatment works.
- A new 250mm diameter pipeline between the lifting pump station and the pressure tower.



Figure 6: Proposed Water Bulk Infrastructure



9 Fire Fighting Requirements

Areas to be protected by a fire service should be classified according to a fire-risk category. The new development can be classified as a “Low risk – Group 4” according to the “Guidelines for Human Settlement Planning and Design”.

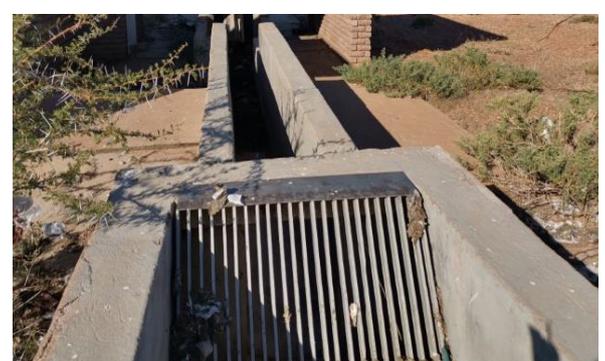
No specific provision for fire fighting water is required in water storage, or reticulation mains in these areas. Hydrants should, however, be located at convenient points in the area on all mains of 75 mm nominal internal diameter and larger, and in the vicinity of all schools, commercial areas and public buildings.

Fire fighting in areas zoned “Low-risk – Group 4” should generally be carried out using trailer-mounted water tanks or fire appliances that carry water, which can be replenished from the hydrants provided in the reticulation, if necessary.

4. SEWERAGE

4.1 Existing Sewage Infrastructure overview

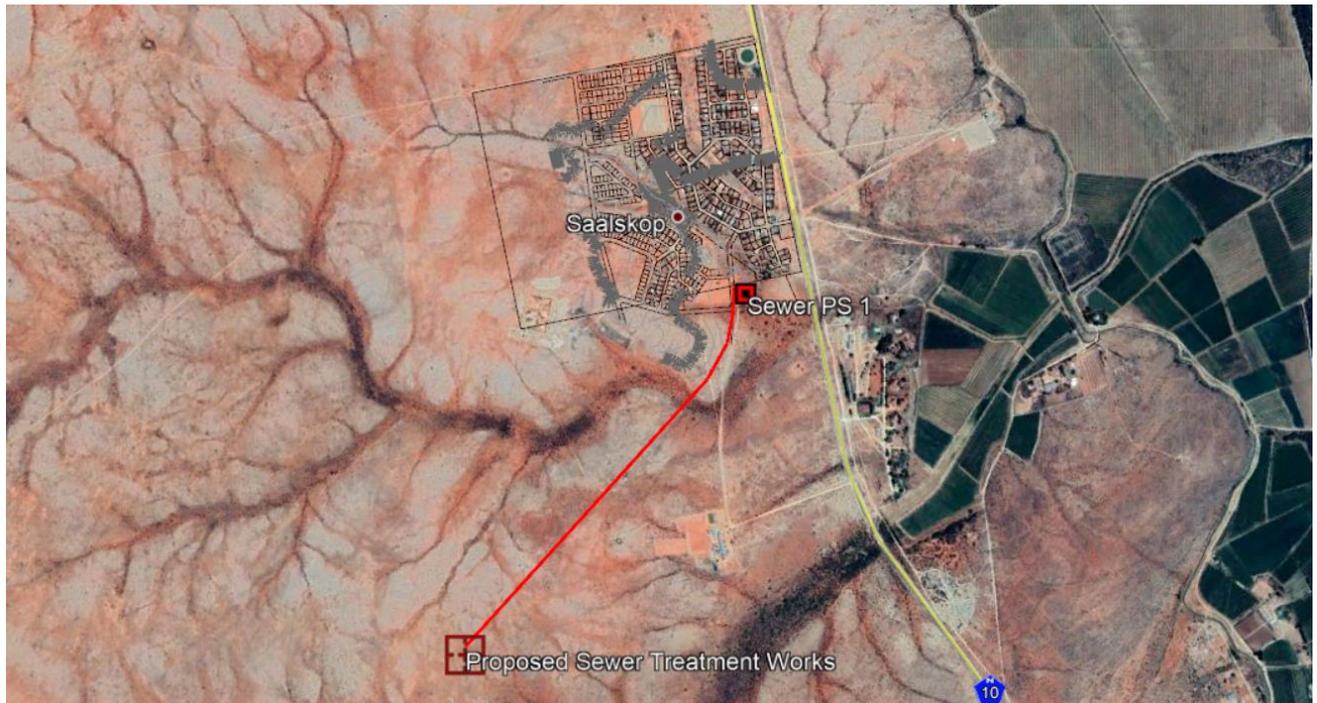
Thirty percent of the houses in the Topline village is currently serviced by VIP toilets. Sixty percent of the houses are using toilets with conservancy tanks which get regularly cleaned by municipal waste truck.





4.2 Bulk Sewer Infrastructure Requirements

If a full borne sewer sewerage system is required for the new 248 houses development, the associated bulk infrastructure will consist of a pumpstation, rising main pipeline and oxidation ponds as shown on the Google image below. The existing Sewer Treatment Works is too close to the edge of the village and needs to be moved to a new location as shown on the Google image below.



The total sewer flow is calculated as follows:

TOPLINE TOTAL SEWER FLOW					
Sewer flow per day - Sub economical houses	536	sub economical houses @	500 l/day	268 000	l/day
Sewer flow per day - Economical houses	0	economical houses @	750 l/day	-	l/day
Sewer flow per day - Hostels	0	persons @	140 l/day	-	l/day
Sewer flow per day - Schools	200	persons @	20 l/day	4 000	l/day
Businesses and State Institutions	0	buildings	100 l/day	-	l/day
SEWER FLOW PER DAY - TOTAL				272 000	l/day



The sizes and capacities of the proposed pump stations and rising mains were calculated as follows:

PUMP STATION No 1 AND RISING MAIN				
Sewer flow per day - Sub economical houses	536	sub economical houses @	500 l/day	268000 l/day
Sewer flow per day - Economical houses		economical houses @	750 l/day	0 l/day
Sewer flow per day - Hostels	0	persons @	140 l/day	0 l/day
Sewer flow per day - Schools	200	persons @	20 l/day	4000 l/day
Businesses and State Institutions	3	buildings	100 l/day	300 l/day
SEWER FLOW PER DAY - TOTAL				272300 l/day
Average sewer flow				3.2 l/s
Factor for inflow from other sources	30%			0.9 l/s
Sewer flow with inflow from other sources				4.1 l/s
PEAK NETWORK SEWER FLOW	2.1		3.5	14.3 l/s
FLOWRATE FROM OTHER PUMP STATIONS				0 l/s
TOTAL PEAK FLOW				14.34 l/s
ACTUAL PUMP ABILITY	1.63	times peak flow		23.4 l/s
Theoretical pump station capacity for normal pump operation	1	hours of peak flow		52 m ³
Theoretical pump station capacity for emergency storage	4	hours of normal flow		59 m ³
TOTAL REQUIRED THEORETICAL PUMP STATION CAPACITY				111 m³
Pump details				kW
Rising main diameter				206 mm
Rising main material				PVC
Rising main length				1650 m
Static pump height				13 m
Friction losses				12 m
Total pump height				25 m

Recommended Topline bulk sewer infrastructure construction (excluding internal sewer lines) are as follows (shown on the drawing above):

- Construction of a new sewer pump station capable of delivering 23.4 l/s direct to the Waste Water Treatment plant.
- New 200mm diameter Class 6 PVC pipelines (2100m & 1300m) between the pump stations and a new Waste Water Treatment Plant (oxidation ponds).
- Construction of a new Waste Water Treatment Plant (oxidation ponds) with a capacity of 0.3MI per day.



5. ROADS AND STORMWATER

5.1 Roads and Access

Access to the development will be from the existing Residential Collector Streets (Class 4b), as shown on the drawing below:

No problems are foreseen regarding roads and access.

5.2 Stormwater Management

The guiding principle underlying the storm water management strategy is that, where possible, the peak run-off from the post-developed site should not exceed that of the pre-developed site for the full range of storm return periods (1:2 to 1:50). Where possible, measures should be incorporated into the site development plan to attenuate the post-development flows to pre-development rates.

The storm water network must be designed to accommodate (flood frequencies as prescribed by “The Red Book”) the minor storm event (1:5 year) in open channels or side drains of streets. The major storm (1:50 year) should be managed through controlled overland flows, above-ground attenuation storage (if required) and berms at the higher end of the site (if required). As no formal storm water system exists in the area, concentration of storm water must be avoided as far as possible. Earthworks on plots should therefore encourage free drainage of the area.

Topline is a small village that generally drains from the centre. Existing roads will be adequate for this purpose.



6. SOLID WASTE

7. ELECTRICAL SUPPLY

7.1 Electrical Demands and Availability

This section of the report covers the availability of the Bulk Electrical connection to the future 248 Community stands, an expected additional load of the proposed development will initially be 298 KVA as per INEP guidelines and the accommodation of this load will form the basis of this report. The community of Topline falls directly under “Eskom Distribution” and the existing electrified homes in the community purchase electricity directly from Eskom and not through the Kheis local Municipality.

The bulk connection to the community / town is via a 22kV overhead line fed from the 10MVA Grobeltershoop sub-station



7.2 Existing Electrical Network

The bulk connection to the community / town is via a 22kV overhead line fed from the Eskom 10MVA Grobeltershoop sub-station , this sub-station is currently in the process of being upgraded to 20MVA and will be commissioned in December 2020.

The existing MV electrical network in the Topline runs through the town via 22 KV overhead line feeder connecting to various pole mounted transformers (see figure 1 below).

The existing feeder can easily handle the future additional 298kVA load only after the upgraded Eskom Groblershoop sub-station is brought online as indicated by Eskom’s network planning department.



	EXISTING POLE TRANSFORMER
	EXISTING 11m WOOD POLE
	EXISTING 9m WOOD POLE
	EXISTING STAY
	EXISTING STRUT POLE
	EXISTING FLYING STAY
	EXISTING MV EARTH MAT
	EXISTING FOX CONDUCTOR
	EXISTING CONDUCTOR TERMINATION
	EXISTING CONDUCTOR JOINT
	EXISTING CUT-OUT FUSES
	FUTURE POLE TRANSFORMER
	PLANNED 11m WOOD POLE
	PLANNED 9m WOOD POLE
	PLANNED STAY
	PLANNED FLYING STAY
	PLANNED MV EARTH MAT
	PLANNED FOX CONDUCTOR
	PLANNED CONDUCTOR TERMINATION
	PLANNED CONDUCTOR JOINT
	TRANSFORMER DESCRIPTION AND CAPACITY
	PLANNED SOLID LINKS
	PLANNED CABLE MARKER
	PLANNED CABLE JOINT
	EXISTING EQUIPMENT TO BE INCORPORATED
	AUTO-RECLOSER INCLUDING FAULT RIDE AND ADVANCED MONITORING AND CONTROL

7.3 Electrical Network Extension

The internal electrical network extension in the Topline community will only be done by Eskom after the formulation processes are completed as this area falls under the Eskom Distribution

8. COST ESTIMATE

The cost estimate for the proposed activities are as provided below. The level of accuracy is commensurate with a concept level design.

Description	Amount
Water Bulk Services	
New mobile 12l/s river pump station	R 850 000,00
0,85km 125mm Ø supply line	R 722 500,00
Upgrading of Water Treatment Works	R 700 000,00
New 360m ³ storage reservoir	R 900 000,00
New 240m ³ storage reservoir	R 840 000,00
New 24l/s lifting pump station	R 240 000,00
0,3km 200mm Ø line from lifting PS to elevated storage	R 285 000,00
Sub-Total (Water)	R 4 537 500,00
Bulk Sewer Services	R -
New 0,25 ML oxidation pond system	R 2 675 662,36
New sewer pump station No 1	R 1 676 508,10
New sewer pump station No 2	R 1 676 508,10
2,1km 110mm Ø uPVC rising main (PS No.1)	R 2 233 596,40
1,3km 110mm Ø uPVC rising main (PS No.2)	R 1 451 837,66
Sub-Total (Sewer)	R 8 262 274,95
Roads and Access	R -
None	R -
Stormwater	R -
None	R -
Electrical	R -
O/H ACSR line ring	R 2 300 000,00
Circuit breaker (11kV, LC1&2)	R 1 550 000,00
O/H ACSR line to POC	R 1 850 000,00
Sub-Total (Electrical)	R 5 700 000,00
Sub-Total	R 18 499 774,95
15% P&G's	R 2 774 966,24
Sub-Total	R 21 274 741,19
10% Contingencies	R 2 127 474,12
Sub-Total	R 23 402 215,31
10% Professional fees	R 2 340 221,53
Sub-Total	R 25 742 436,84
15% VAT	R 3 861 365,53
Grand Total	R 29 603 802,37

Notes:

- 1) Base date of the calculations is April 2020;
- 2) No provision was made for EIA, registration and/or land acquisition;
- 3) No allowance was made for institutional and/or social development.



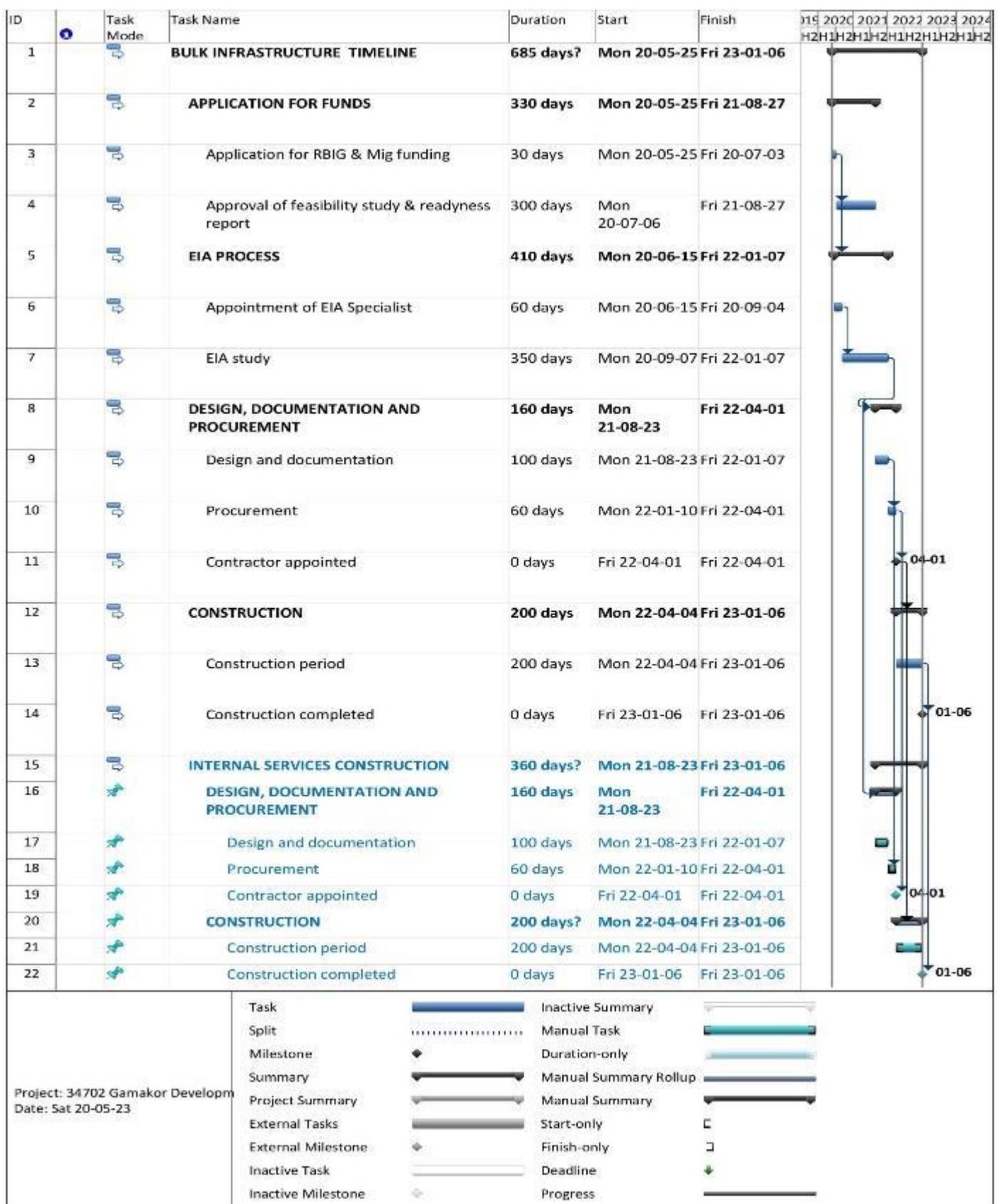
8.1 Funding

Funding can be applied for through the Municipal Infrastructure Grant (MIG) and Regional Bulk Infrastructure Grant (RBIG). For repair work at the water treatment works, the Water and Sanitation Infrastructure Grant (WSIG) can also be applied for.

This report can be used for funding application from the various schemes available.



9. PROJECT TIMELINE



10. CONCLUSION

Engineering services were assessed to determine spare capacity on the existing bulk infrastructure and compared to the estimated demand of the newly proposed Topline 248 houses development.

The findings and conclusions in this report are based on a preliminary desktop study, as well as site visits.

- Bulk Water Infrastructure – The current capacity of the bulk water infrastructure is not enough to accommodate the proposed 248 houses development as is. It is proposed that the infrastructure should be upgraded, not only to provide adequate capacity for the Topline development, but also for future water demand increases. The following upgrades are proposed:
 - Construction of a new 12l/s mobile river pump station with a duty and standby pump.
 - New 125mm diameter Class 6 PVC pipeline between the river pump station and the existing potable water storage reservoir.
 - Upgraded Water Treatment Works capable of delivering 24m³/h on the existing treatment works site
 - A new 360m³ sectional steel reservoir next to the upgraded water treatment works
 - A new 250m³ sectional steel pressure tower on the highest point to the north.
 - A new 24l/s uplifting pump station at the treatment works.
 - A new 200mm pipeline between the lifting pump station and the pressure tower.
- Bulk Sewage Infrastructure – There is currently no bulk sewer infrastructure. Recommended Topline bulk sewer infrastructure construction (excluding internal sewer lines) are as follows (shown on the drawing above):
 - Construction of two new sewer pump stations capable of delivering 6.7 l/s direct to the Waste Water Treatment plant.
 - New 110mm diameter Class 6 PVC pipelines (2100m & 1300m) between the pump stations and a new Waste Water Treatment Plant (oxidation ponds).
 - Construction of a Waste Water Treatment Plant (oxidation ponds) with a capacity of 0.5MI per day.
- Roads and Access: No bulk infrastructure upgrading required on the roads.
- Storm Water Management: No bulk infrastructure upgrading required on the storm water.
- Electricity Supply – Formal bulk upgrade process to be finalised between Eskom and !Kheis Municipality.
- Electrical Load Centre – The existing Load Centre “Topline Nommer 2” can accommodate the future additional load, with only minor modification to be done in the Load Centre and as agreed with the Municipality’s Electrical Department.



In conclusion, the engineering services are not in place (water and sewer) to meet the standard requirements. The infrastructure will have to be upgraded regardless of the implementation of the Topline 248 houses development in order to meet current and expected future needs. The upgrading should be done in such a way as to take into consideration the Topline 248 Houses development.